

**Exercise 1:** The Distortion Energy Criterion of yielding assumes that yielding starts when the distortion energy at a point in a solid becomes equal to the distortion energy at yield in simple tension of the same material.

- (1) Show that the energy of distortion per unit volume for the general loading can be expressed as,

$$W_d^*(\sigma_{ij}) = \frac{1+\nu}{6E} \left[ (\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2 + 6(\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2) \right] \quad (a)$$

- (2) If the yield stress of the material in uniaxial tension is  $\sigma_Y$  show that this criterion is expressed as,

$$\sigma_Y = \frac{1}{\sqrt{2}} \left[ (\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2 + 6(\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2) \right]^{1/2} \quad (b)$$

*Solution*

The strain energy density in terms of stresses is,

$$W^*(\sigma_{ij}) = \frac{1}{2E} \left[ \sigma_{11}^2 + \sigma_{22}^2 + \sigma_{33}^2 - 2\nu(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11}) \right] + \frac{1}{2\mu} (\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2) \quad (c)$$

$$W^*(\sigma_{ij}) = W_d^*(\sigma_{ij}) + W_p^*(\sigma_{ij}) \quad (d)$$

$$W_p^*(\sigma_{ij}) = 3 \frac{1}{2} \left( \frac{1}{3} \varepsilon_{ii} \frac{1}{3} \sigma_{ii} \right) \quad (e)$$

From Hook's law,

$$\varepsilon_{ii} = \frac{1-2\nu}{E} \sigma_{ii}$$

$$W_p^*(\sigma_{ij}) = 3 \frac{1}{2} \left[ \left( \frac{1-2\nu}{3E} \sigma_{ii} \right) \left( \frac{1}{3} \sigma_{kk} \right) \right] = \frac{1-2\nu}{6E} \sigma_{kk} \sigma_{ii} = \frac{1-2\nu}{6E} (\sigma_{11} + \sigma_{22} + \sigma_{33})^2$$

$$W_d^*(\sigma_{ij}) = W^*(\sigma_{ij}) - W_p^*(\sigma_{ij})$$

$$W_d^*(\sigma_{ij}) = \frac{1}{2E} \left[ \sigma_{11}^2 + \sigma_{22}^2 + \sigma_{33}^2 - 2\nu(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11}) \right] + \frac{1}{2\mu} (\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2) - \frac{1-2\nu}{6E} (\sigma_{11} + \sigma_{22} + \sigma_{33})^2 \quad (f)$$

$$W_d^*(\sigma_{ij}) = \frac{1}{2E} \left[ \sigma_{11}^2 + \sigma_{22}^2 + \sigma_{33}^2 - 2\nu(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11}) \right] - \frac{1-2\nu}{6E} (\sigma_{11} + \sigma_{22} + \sigma_{33})^2$$

$$+ \frac{1}{2\mu} (\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2)$$

$$W_d^*(\sigma_{ij}) = \frac{1}{6E} \left[ 3\sigma_{11}^2 + 3\sigma_{22}^2 + 3\sigma_{33}^2 - 6\nu(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11}) \right] +$$

$$\frac{1}{6E} \left[ -(1-2\nu)(\sigma_{11}^2 + \sigma_{22}^2 + \sigma_{33}^2 + 2(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11})) \right] + \frac{1}{2\mu} (\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2)$$

Combing the terms in the first two brackets, and replacing  $\mu = E / 2(1 + \nu)$  we obtain

$$W_d^*(\sigma_{ij}) = \frac{1+\nu}{6E} \left[ (\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2 + 6(\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2) \right]$$

In uniaxial tension we have

$$W_d^*(\sigma_{ij}) = \frac{1+\nu}{6E} [2\sigma_{11}^2] = \frac{1+\nu}{6E} [2\sigma_Y^2]$$

Setting equal the right hand side of the two last expressions, we obtain (b).

**Exercise 2:** The stress state at a point of a solid is

$$\sigma_{ij} = \begin{pmatrix} \sigma & \tau & 0 \\ \tau & \sigma & 0 \\ 0 & 0 & \sigma \end{pmatrix}$$

where  $\sigma, \tau$  are given stress. What is the yield condition according to (a) Tresca and (b) V Mises criteria?

*Solution*

From the given stress matrix we can see that one of the principal stresses is  $\sigma$  and the other two are

$$\sigma_{1,2} = \frac{\sigma + \sigma}{2} \pm \sqrt{\left(\frac{\sigma - \sigma}{2}\right)^2 + \tau^2} = \sigma \pm \tau$$

$$\Rightarrow \sigma_1 = \sigma + \tau, \sigma_2 = \sigma, \sigma_3 = \sigma - \tau$$

(a) criterion

$$\sigma_1 - \sigma_3 = 2\tau = \sigma_Y \Rightarrow \tau = \sigma_Y / 2$$

(b) V Mises criterion

$$\sigma_Y = \frac{1}{\sqrt{2}} \left[ (\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2 \right]^{1/2} = \sqrt{3}\tau$$

$$\Rightarrow \tau = \sigma_Y / \sqrt{3}$$

**Exercise 3:** A thick-walled cylinder, with open ends, internal radius  $r_i$  and external radius  $2r_i$  is subjected to internal pressure  $P_i$ . The tensile yield stress of the material is  $\sigma_Y$ . Determine the internal pressure at the onset of yielding using the Tresca and V Mises yield criteria. Calculate the displacement at the onset of yielding at the internal surface of the cylinder (modulus of elasticity and Poisson ratio  $E, \nu$  are known).

*Solution*

The maximum stresses are at the internal surface of the cylinder. They are given by,

$$\sigma_{rr} = \frac{1}{r_e^2 - r_i^2} \left( r_i^2 P_i - r_e^2 P_e + \frac{r_i^2 r_e^2}{r^2} (P_e - P_i) \right)$$

$$\sigma_{\theta\theta} = \frac{1}{r_e^2 - r_i^2} \left( r_i^2 P_i - r_e^2 P_e - \frac{r_i^2 r_e^2}{r^2} (P_e - P_i) \right)$$

For  $P_e = 0$  and the given radii, we obtain

$$\sigma_{rr} = \frac{1}{(2r_i)^2 - r_i^2} \left( P_i r_i^2 - \frac{P_i r_i^2 (2r_i)^2}{r_i^2} \right) = \frac{P_i r_i^2}{(2r_i)^2 - r_i^2} (1 - 4) = -P_i$$

$$\sigma_{\theta\theta} = \frac{1}{(2r_i)^2 - r_i^2} \left( P_i r_i^2 + \frac{P_i r_i^2 (2r_i)^2}{r_i^2} \right) = \frac{P_i r_i^2}{(2r_i)^2 - r_i^2} (1 + 4) = \frac{5}{3} P_i$$

They are also principal stresses.

V Mises criterion,

$$2\sigma_Y^2 = \left( -P_i - \frac{5P_i}{3} \right)^2 + \left( \frac{5P_i}{3} \right)^2 + (P_i)^2$$

$$2\sigma_Y^2 = \left( \frac{8P_i}{3} \right)^2 + \left( \frac{5P_i}{3} \right)^2 + (P_i)^2 \Rightarrow P_i = \sqrt{\frac{18}{64 + 25 + 9}} \sigma_Y = 0.428 \sigma_Y$$

Tresca Criterion

$$\sigma_{\theta\theta} - \sigma_{rr} = \frac{5}{3} P_i - (-P_i) = \sigma_Y \Rightarrow P_i = \frac{3}{8} \sigma_Y = 0.375 \sigma_Y$$

Displacement

$$u_r = \frac{r_i^2 P_i r}{E(r_e^2 - r_i^2)} \left[ (1 - \nu) + (1 + \nu) \frac{r_e^2}{r^2} \right] = \frac{r_i^2 P_i r_i}{E((2r_i)^2 - r_i^2)} \left[ (1 - \nu) + (1 + \nu) \frac{(2r_i)^2}{r_i^2} \right]$$

$$= \frac{P_i r_i}{3E} [(1 - \nu) + 4(1 + \nu)] = \frac{5 + 3\nu}{3} \frac{P_i r_i}{E}$$

**Exercise 4:** Express the plastic strain increment ratios for

- (1) Simple tension  $\sigma_{11} = \sigma_Y$
- (2) Biaxial stress with  $\sigma_{11} = -\sigma_Y / \sqrt{3}$ ,  $\sigma_{22} = \sigma_Y / \sqrt{3}$ ,  $\sigma_{33} = \sigma_{12} = \sigma_{23} = \sigma_{13} = 0$
- (3) Pure shear  $\sigma_{12} = \sigma_Y$

*Solution*

The plastic strain increment ratios are given by,

$$\frac{d\varepsilon_1^p}{s_1} = \frac{d\varepsilon_2^p}{s_2} = \frac{d\varepsilon_3^p}{s_3} = d\lambda \quad (\text{Appendix C}) \quad (\text{C.24a})$$

Here  $s_i$  ( $i=1,2,3$ ) are the principal values of the deviatoric stress tensor.

(1)

$$\begin{aligned} \sigma_{11} &= \sigma_1 = \sigma_Y, \quad \sigma_2 = \sigma_3 = 0 \\ \Rightarrow s_1 &= \sigma_Y - \frac{1}{3}\sigma_Y = \frac{2}{3}\sigma_Y, \quad s_2 = s_3 = \sigma_2 - \frac{1}{3}\sigma_1 = -\frac{1}{3}\sigma_Y \\ \Rightarrow \frac{d\varepsilon_1^p}{2} &= \frac{d\varepsilon_2^p}{-1} = \frac{d\varepsilon_3^p}{-1} \end{aligned}$$

(2)

$$\begin{aligned} \sigma_{11} &= \sigma_1 = \sigma_Y / \sqrt{3}, \quad \sigma_2 = 0, \quad \sigma_3 = -\sigma_Y / \sqrt{3}, \\ \Rightarrow s_1 &= \sigma_Y / \sqrt{3}, \quad s_2 = 0, \quad s_3 = -\sigma_Y / \sqrt{3} \\ \Rightarrow \frac{d\varepsilon_1^p}{1} &= \frac{d\varepsilon_3^p}{-1} \end{aligned}$$

The term with the second component is considered zero because when the denominator is zero, the numerator is taken as zero in the theory.

(3) In simple shear we have  $\sigma_{12} = \sigma_Y$

$$\begin{aligned} \sigma_1 &= \sigma_Y, \quad \sigma_2 = 0, \quad \sigma_3 = -\sigma_Y \quad \Rightarrow s_1 = \sigma_Y, \quad s_2 = 0, \quad s_3 = -\sigma_Y \\ \Rightarrow \frac{d\varepsilon_1^p}{1} &= \frac{d\varepsilon_3^p}{-1} \end{aligned}$$

**Solutions of problems from a previous examination**

**Problem A:**

**Stresses:** We have here a problem of plane stress. We assume the following stress field,

$$\sigma_{11} = \sigma_{11}(x_2), \quad \sigma_{22} = \sigma_{12} = 0 \quad (a)$$

This stress field satisfies the equilibrium equations,

$$\frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{12}}{\partial x_2} = 0; \quad \frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{22}}{\partial x_2} = 0 \quad (b)$$

The compatibility equation is,

$$\left( \frac{\partial^2}{\partial x_1^2} + \frac{\partial^2}{\partial x_2^2} \right) (\sigma_{11} + \sigma_{22} + \alpha ET) = 0 \Rightarrow \frac{\partial^2}{\partial x_2^2} (\sigma_{11} + \alpha ET) = 0$$

$$\text{Integrating we obtain, } \sigma_{11} + \alpha ET = c_1 x_2 + c_2 \Rightarrow \sigma_{11} = -\alpha ET + c_1 x_2 + c_2 \quad (c)$$

$$\textbf{Strains: } \varepsilon_{11}(x_2) = \frac{\sigma_{11}(x_2)}{E} + \alpha T(x_2); \quad \varepsilon_{22} = \frac{-\nu \sigma_{11}(x_2)}{E} + \alpha T(x_2); \quad \varepsilon_{12} = 0$$

**BC:** At the free ends we have the normal force and moment both equal zero,

$$\int_{-h}^{+h} \sigma_{11}(x_2) t dx_2 = 0; \quad \int_{-h}^{+h} \sigma_{11}(x_2) x_2 t dx_2 = 0 \quad (d)$$

Substituting (c) in (d) we obtain the constants,

$$c_1 = \frac{3}{2h^3} \int_{-h}^{+h} \alpha ET x_2 dx_2; \quad c_2 = \frac{1}{2h} \int_{-h}^{+h} \alpha ET dx_2 \Rightarrow \sigma_{11} = \alpha E \left( -T + \frac{t}{2ht} \int_{-h}^{+h} T dx_2 + \frac{3tx_2}{2th^3} \int_{-h}^{+h} T x_2 dx_2 \right)$$

$$\text{Define } A = 2ht, \quad I_3 = \frac{2h^3 t}{3} \text{ then, } \Rightarrow \sigma_{11} = \alpha E \left( -T + \frac{t}{A} \int_{-h}^{+h} T dx_2 + \frac{tx_2}{I_3} \int_{-h}^{+h} T x_2 dx_2 \right)$$

Note that when  $T = \text{const}$ , the stress is zero because the second integral is zero and the first one is equal to  $-T$ .

**Problem B:**

1: Because of the parallel arrangement,  $\sigma = \sigma_e + \sigma_v$ .  $\varepsilon = \varepsilon_e = \varepsilon_v$

For the spring  $\sigma_e = E\varepsilon_e$ ; For the dashpot  $\sigma_v = \eta \frac{d\varepsilon_v}{dt} = \eta \dot{\varepsilon}_v$

We add these stresses and set the strains equal we obtain for the Kelvin-Voigt model,

$$\sigma = E\varepsilon + \eta \dot{\varepsilon}$$

2: With the configuration of the elements in the Figure we can write,

$$\varepsilon_1 + \varepsilon_2 = \varepsilon \quad (a1)$$

$$\sigma_1 = \sigma_2 = \sigma \quad (a2)$$

$$\sigma_1 = \sigma = \eta_1 \dot{\varepsilon}_1 \quad \text{for the dashpot} \quad (b)$$

$$\sigma_2 = \sigma = E_2 \varepsilon_2 + \eta_2 \dot{\varepsilon}_2 \quad \text{for the Kelvin-Voigt} \quad (c)$$

From (a1) and (b) we can write,

$$\dot{\varepsilon}_2 = \dot{\varepsilon} - \dot{\varepsilon}_1 = \dot{\varepsilon} - \frac{\sigma_1}{\eta_1}; \Rightarrow \ddot{\varepsilon}_2 = \ddot{\varepsilon} - \frac{\dot{\sigma}_1}{\eta_1} \quad (d)$$

Introduce (d) in (c) and rewrite,

$$\dot{\sigma} = E_2 \left( \dot{\varepsilon} - \frac{\sigma}{\eta_1} \right) + \eta_2 \left( \ddot{\varepsilon} - \frac{\dot{\sigma}}{\eta_1} \right) \quad \text{or} \quad \dot{\sigma} \left( \frac{\eta_1 + \eta_2}{E_2} \right) + \sigma = \frac{\eta_1 \eta_2}{E_2} \ddot{\varepsilon} + \eta_1 \dot{\varepsilon}$$